

Groundwater Transfer Review Summary Form

Transfer/PA # T- 14646

GW Reviewer Gabriela Ferreira Date Review Completed: July 18, 2025

Summary of Same Source Review:

☐ The proposed change in point of appropriation is not within the same aquifer as per OAR 690-380-2110(2).

Summary of Water Level Decline Condition Review:

☐ Water levels at the original point(s) of appropriation have exceeded the allowed decline threshold defined by conditions in the originating water right.

Summary of Injury Review:

☐ The proposed transfer will result in another, existing water right not receiving previously available water to which it is legally entitled or result in significant interference with a surface water source as per 690-380-0100(3).

Summary of GW-SW Transfer Similarity Review:

☐ The proposed SW-GW transfer doesn't meet the definition of "similarly" as per OAR 690-380-2130.

This is only a summary. Documentation is attached and should be read thoroughly to understand the basis for determinations.



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Ground Water Review Form:

- ☒ **Water Right Transfer**
☐ **Permit Amendment**
☐ **GR Modification**
☐ **Other**

Application: T-14646

Applicant Name: Hillsboro Landfill, Inc.

Proposed Changes: ☐ POA ☒ APOA ☐ SW→GW ☐ RA
☐ USE ☐ POU ☐ OTHER

Reviewer(s): Gabriela Ferreira

Date of Review: July 18, 2025

Date Reviewed by GW Mgr. and Returned to WRSD: _____

The information provided in the application is insufficient to evaluate whether the proposed transfer may be approved because:

- ☐ The water well reports provided with the application do not correspond to the water rights affected by the transfer.
- ☐ The application does not include water well reports or a description of the well construction details sufficient to establish the ground water body developed or proposed to be developed.
- ☐ Other _____

Basic description of the changes proposed in this transfer: The proposed transfer relates to wells located in Hillsboro, Oregon. The proposed transfer would modify **Certificate 96345**, which currently authorizes **0.36 cfs** (162 gpm) by two existing wells, **WASH 1130** (NW3) and **WASH 9603** (NW5) for Industrial use. The two authorized POAs are also associated with Certificate 87500 and Permit G-12247.

The proposed transfer would add one APOA, **WASH 9628** (NW6), altered under WASH 119 to replace the original 6" casing with 8" casing. The proposed APOA is located approximately 450 feet southeast of currently authorized POA WASH 1130. All three wells were the authorized POAs on two previous limited licenses: LL-1394, expired 2/28/2017; and LL-1747, expired 10/31/2022.

1. Will the proposed POA develop the same aquifer (source) as the existing authorized POA?
☒ Yes ☐ No Comments: The authorized POAs WASH 1130 and WASH 9603 are completed to depths of 209 and 210 feet below ground surface (bgs), respectively. Proposed APOA WASH 9628 is completed to a depth of 228 feet bgs. All three wells produce from relatively-thin (5-20 ft thick) sand and gravel beds interlayered with lower-permeability silts and clays that are present from ground surface to approximately 1200 feet below ground surface (Gannett and Caldwell, 1998).
2. a) Is the existing authorized POA subject to a water level decline condition?
☐ Yes ☒ No Comments: _____

b) If yes, for each POA identify the reference level, most recent spring-high water level, and whether an applicable permit decline condition has been exceeded: _____

3. a) Is there more than one source developed under the right (e.g., basalt and alluvium)?

☐ Yes ☒ No Comments: _____

b) If yes, estimate the portion of the right supplied by each of the sources and describe any limitations that will need to be placed on the proposed change (rate, duty, etc.): _____

4. a) Will this proposed change, at its maximum allowed rate of use, likely result in an increase in interference with **another ground water right**?

☒ Yes ☐ No Comments: Authorized POA WASH 1130 is located approximately 1,010 feet from WASH 9658, an authorized POA for Certificate 35603. The proposed APOA WASH 9628 would be located 670 feet from WASH 9658. The reduced intervening distance proposed by this change would likely result in an increase in interference with WASH 9658.

b) If yes, would this proposed change, at its maximum allowed rate of use, likely result in another groundwater right not receiving the water to which it is legally entitled?

☐ Yes ☒ No If yes, explain: The potential change in interference to WASH 9658 from Proposed APOA WASH 9628 as compared to WASH 1130 was evaluated with the maximum authorized rate of 162 gpm. The results are shown on the attached figures.

The reduced intervening distance between proposed APOA WASH 9628 and WASH 9658 is estimated to potentially increase interference by 3-5 feet under likely scenarios. Therefore, the proposed change is unlikely to result in another groundwater right not receiving the water to which it is legally entitled.

5. a) Will this proposed change, at its maximum allowed rate of use, likely result in an increase in interference with **another surface water source**?

☐ Yes ☒ No Comments: The proposed POA WASH 9628 is approximately 3,700 feet west of the Tualatin River, whereas the authorized POAs WASH 1130 and WASH 9603 are 4,100 – 4,500 feet west of the Tualatin River. Although water-bearing zones are identified at depths greater than 90 feet bgs, groundwater may be hydraulically connected to the Tualatin River at greater distances. However, additional pumping stresses at the proposed POA will be attenuated by the distance and low-permeability sediments above water-bearing zones; therefore, there is limited effective hydraulic connection between the aquifer system and the local stream network within the distances considered for this review.

b) If yes, at its maximum allowed rate of use, what is the expected change in degree of interference with any **surface water sources** resulting from the proposed change?

Stream: _____

☐ Minimal ☐ Significant

Stream: _____

☐ Minimal ☐ Significant

Provide context for minimal/significant impact: None

6. For SW-GW transfers, will the proposed change in point of diversion affect the surface water source similarly (as per OAR 690-380-2130) to the authorized point of diversion specified in the water use subject to transfer?

☐ Yes ☐ No Comments: N/A

What conditions or other changes in the application are necessary to address any potential issues identified above: None.

Any additional comments: None.

References:

Application T-14646 File

Well reports and static water level information: WASH 1103, WASH 9603, WASH 9628,
WASH 9658

Gannett, M.W. and Caldwell, R., 1998, Geologic framework of the Willamette Lowland aquifer system, Oregon and Washington: U.S. Geological Survey Professional Paper 1424-A, 32 p.

The map displays the Minter Bridge area in Hillsboro, Oregon, with the Tualatin River flowing through it. Key features include:

- Streets:** SE Bianca St, SE Roundelay St, SE Fir, SE Grove Loop, SE Minter Bridge Rd, SE Jean Ln, SE Curtin Dr, SE Quail Ct, SE Tanager Cir, SE Road Bridge Dr, SE Blue Bird Dr, SE Robin Ct, SE Smith Dr, SE Singing Woods Dr, SE Noland St, SE Cypress St, SE Rosespring Dr, SE 30th Ave, SE 24th Ave, SE Meadow Lark Dr, SE 25th Ct, SE 30th Ct, SE Timber, SE Road Bridge Rd, SE Waik, SE 30th Ave, SE 24th Ave, SE Meadow Lark Dr, SE 25th Ct, SE 30th Ct, SE Timber.
- Landmarks:** Jackson Bottom Wetlands, Minter Bridge Elementary, Hillsboro High.
- Proposed and Authorized Points:**
 - Proposed APOA:** Located near SE Minter Bridge Rd and SE Noland St.
 - Authorized POA:** Two locations are marked with yellow dots and labeled: one near SE Minter Bridge Rd and SE Noland St, and another near SE Minter Bridge Rd and SE Noland St.
- Map Details:** The map includes a scale bar (0 to 5,280 feet) and a north arrow. The Tualatin River is shown in blue, and the surrounding area is in shades of green and brown.

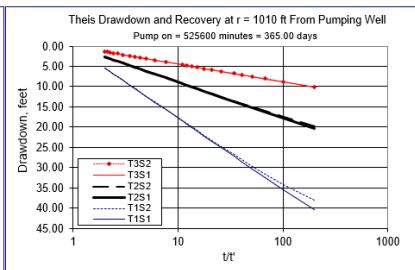
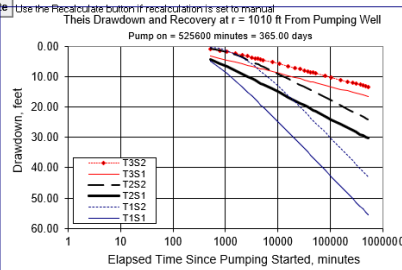
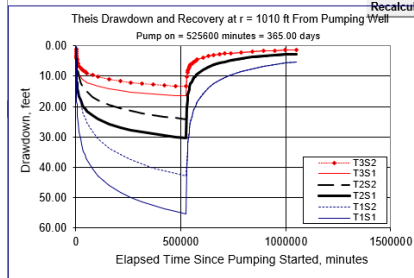
Service Layer Credits: Sources: Esri, HERE, Garmin, USGS, Intermap, INCREMENT P, NRCan, Esri Japan, METI, Esri China (Hong Kong), Esri Korea, Esri (Thailand), NGCC, (c) OpenStreetMap contributors, Swatch

Modeled drawdown for WASH 9658 as a result of pumping Authorized POA WASH 1130

Theis Time-Drawdown Worksheet v.3.00

Calculates Theis nonequilibrium drawdown and recovery at any arbitrary radial distance, r , from a pumping well for 3 different T values and radial distance, r , from a pumping well for 3 different T values and 2 different S values.
Written by Karl C. Wozniak September 1992. Last modified December 30, 2014

| Input Data: | Var Name | Scenario 1 | Scenario 2 | Scenario 3 | Units | |
|-----------------------------------|----------|------------|------------|------------|---------|---------------|
| Total pumping time | t | | 365 | | d | |
| Radial distance from pumped well: | r | | 1010.00 | | ft | |
| Pumping rate | Q | | 0.360 | | cfs | 161.57 gpm |
| Hydraulic conductivity | K | 8.000 | 16.000 | 32.000 | ft/day | 0.36 cfs |
| Aquifer thickness | b | | 40 | | ft | 21.60 cfm |
| Storativity | S_1 | | 0.00020 | | | 31,104.00 cfd |
| | S_2 | | 0.00100 | | | 0.71 af/d |
| Transmissivity Conversions | | | | | | |
| | T_ft2pd | 320 | 640 | 1,280 | ft2/day | |
| | T_ft2pm | 0.2222 | 0.4444 | 0.8889 | ft2/min | |
| | T_gpd/ft | 2.394 | 4.787 | 9.574 | gpd/ft | |



Modeled drawdown for WASH 9658 as a result of pumping Proposed APOA 9628

Theis Time-Drawdown Worksheet v.3.00

Calculates Theis nonequilibrium drawdown and recovery at any arbitrary radial distance, r , from a pumping well for 3 different T values and radial distance, r , from a pumping well for 3 different T values and 2 different S values.
Written by Karl C. Wozniak September 1992. Last modified December 30, 2014

| Input Data: | Var Name | Scenario 1 | Scenario 2 | Scenario 3 | Units | |
|-----------------------------------|----------|------------|------------|------------|---------|---------------|
| Total pumping time | t | | 365 | | d | |
| Radial distance from pumped well: | r | | 670.00 | | ft | |
| Pumping rate | Q | | 0.360 | | cfs | 161.57 gpm |
| Hydraulic conductivity | K | 8.000 | 16.000 | 32.000 | ft/day | 0.36 cfs |
| Aquifer thickness | b | | 40 | | ft | 21.60 cfm |
| Storativity | S_1 | | 0.00020 | | | 31,104.00 cfd |
| | S_2 | | 0.00100 | | | 0.71 af/d |
| Transmissivity Conversions | | | | | | |
| | T_ft2pd | 320 | 640 | 1,280 | ft2/day | |
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