Groundwater Application Review Summary Form

Application # G- <u>18840</u>	
GW Reviewer Travis Brown	Date Review Completed: 8/28/2019
Summary of GW Availability and Injury Review:	
[] Groundwater for the proposed use is either over a amounts requested without injury to prior water right capacity of the groundwater resource per Section B of the groundwater per Section B of the	nts, OR will not likely be available within the
Summary of Potential for Substantial Interference R	Review:
(X) There is the potential for substantial interference	per Section C of the attached review form.
Summary of Well Construction Assessment:	
[] The well does not appear to meet current well coreview form. Route through Well Construction and Constru	

This is only a summary. Documentation is attached and should be read thoroughly to understand the basis for determinations and for conditions that may be necessary for a permit (if one is issued).

WATER RESOURCES DEPARTMENT

MEM	O						_	Augus:	+ 28	,20 <u>/</u> 9	3
TO:		Applica									
FROM	M:	GW :	Travis Reviewer				-				
SUBJ	SUBJECT: Scenic Waterway Interference Evaluation										
	YES	Th	ne source of appropriation is within or above a Scenic Waterway								
×	NO	The sou	irce of a	.ppropri	ation is	WILIIII C	n above	a Scen	ic wate.	iway	
	YES	TI Ab	Jse the Scenic Waterway condition (Condition 7J)								
	NO	Use the									
	interfe	RS 390. rence wated inter	ith sur	face wa	ater tha	t contri				_	
	interfe the De that t	RS 390.8 rence we epartme che propartme cary to n	ith surfa ent is un posed	ace wate nable to use wil	er that of the find the find the find the find the find the first	contribu hat the urably	tes to a re is a reduce	scenic prepone the s	waterwa deranc urface	ay; ther e of evi water	efore, idence
DISTRIBUTION OF INTERFERENCE Calculate the percentage of consumptive use by month and fill in the table below. If interference cannot be calculated, per criteria in 390.835, do not fill in the table but check the "unable" option above, thus informing Water Rights that the Department is unable to make a Preponderance of Evidence finding.											
Water	cise of th rway by n surface	the follo	owing a	mounts			-		e consu		Scenic use by
Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec

PUBLIC INTEREST REVIEW FOR GROUNDWATER APPLICATIONS

TO:			Rights Se							J	Date <u>8/2</u>	8/2019
FROM:	:	Groun	dwater Se	ction		Travis E	Brown					
CLIDIE	CT	A 11		0040			ver's Name					
SUBJE	CT:	Applic	cation G- <u>1</u>	8840		Supe	ersedes	review of		Date of Rev	• ()	
										Date of Rev	iew(s)	
PUBLI	C INTE	REST	PRESUN	APTION; (GROUNE)WATER	!					
								water use will en	sure the pr	eservation of	the publi	ic
								ew groundwater				
								s the proposed us				
								nd agency polici				
the press	amption c			n is suscu u	pon avano			nd agency pone	es in place	ar the time	01 0 / 4144	
A. <u>GEN</u>	NERAL 1	INFO:	RMATIO	<u>N</u> : App	plicant's N	ame: N	<u> Iayfield</u>	Farms, LLC		County:]	MARION	1
A1.	1. Applicant(s) seek(s) <u>0.45</u> cfs from <u>5</u>				well(s)) in the _	Willamette				Basin,	
	M	ainster	n Willamet	te		subbas	sin					
A2.	Proposed	use	Com	mercial		Seaso	nality: _	Year-round				
A3.	Well and	aquife	r data (atta	ch and nun	ıber logs fo	or existing	wells; n	nark proposed v	vells as suc	h under logi	(d):	
*** 11	<u> </u>	,	Applicant'	's B	1.4 .6	Propo	sed	Location	Loc	cation, metes a	and bounds	s, e.g.
Well	Logic	1	Well #	Propose	ed Aquifer*	Rate(c		(T/R-S QQ-Q		50' N, 1200' E		
1	MARI 68		1		luvium	0.45		3S/1W-32 SE-S		335' N, 1165' W fr SE cor		
2	Propose		3		luvium	0.45		3S/1W-32 SE-S		20' N, 930' W fr SE cor S 32 25' N, 765' W fr SE cor S 32		
3 4	Propose Propose		4		luvium luvium	_	0.45 3S/1W-32 SE-SE 0.45 3S/1W-32 SE-SE			45' N, 565' W		
5	Propose		5		luvium	0.45 3S/1W-32 SE-SE 0.45 3S/1W-32 SE-SE				960' N, 580' W		
	ım, CRB, B									,		
	T						1	1			T	,
337 11	Well	First	1 \ \W/1	SWL	Well	Seal Casing Liner Per					Draw	Test
Well	Elev ft msl	Wate ft bls	I II his	Date	Depth (ft)	Interval	Interva	als Intervals (ft)	Or Screer (ft)	ns Yield (gpm)	Down (ft)	Type
1	~196ª	185	96	8/24/2018	217	(ft) (ft) 0-120 +2-16			167-207		(11)	Air
							207-21	17	(Screen)			
		l l			220 ^b	0-120b	0-220		TBD ^b			
2	~192ª				220 ^b	0-120 ^b	0-220		TBD^b			
3	~193ª				220b	0-120°	0-220	b	TRD^b			
					220 ^b	0-120 ^b 0-120 ^b	0-220 0-220		TBD ^b			
3 4 5	~193 ^a ~193 ^a ~189	cation fe	or proposed v	wells.								
3 4 5	~193 ^a ~193 ^a ~189	cation fo	or proposed v	wells.								
3 4 5	~193ª ~193ª ~189 from applie	nts: Th	ne proposed	POA/POU a	220 ^b	0-120 ^b mately 0.5 i	0-220		TBD^b	nunity of Bu	tteville, O	regon.
3 4 5 Use data	~193ª ~193ª ~189 from applie	nts: Th	ne proposed		220 ^b	0-120 ^b mately 0.5 i	0-220	b	TBD^b	nunity of Bu	tteville, O	regon.
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3 4 5 Use data	~193a ~193a ~189 from applio Commer No volum a LIDAR	nts: <u>Th</u> netric l elevati	ne proposed imit on the ion at existi	POA/POU a proposed us ng/proposed	are approxime has been well locate	0-120b mately 0.5 p specified.	0-220	b	TBD^b	nunity of Bu	tteville, O	regon.
3 4 5 Use data	~193a ~193a ~189 from applio Commer No volum a LIDAR	nts: <u>Th</u> netric l elevati	ne proposed imit on the ion at existi	POA/POU a proposed us	are approxime has been well locate	0-120b mately 0.5 p specified.	0-220	at of the unincorp	TBD^b	munity of Bu	tteville, O	regon.
3 4 5 Use data	~193a ~193a ~189 from applio Commer No volum a LIDAR	nts: <u>Th</u> netric l elevati	ne proposed imit on the ion at existi	POA/POU a proposed us ng/proposed	are approxime has been well locate	0-120b mately 0.5 p specified.	0-220	at of the unincorp	TBD^b	munity of Bu	tteville, O	regon.
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3 4 5 Use data A4.	romapplica relationship in the commer relationship in the comment of the comment	elevation of the construction of the construct	ne proposed imit on the ion at existi struction from the groundwater les contain e proposed nt Willamet istrative are	POA/POU a proposed us ng/proposed om applicatio Will er hydraulica such provis. POA are gra te Basin rule a: N/A	220b are approxime has been well located by the lo	mately 0.5 is specified. ion (Water ted to surfated wile from 500-502-005	miles eas shed Sci Basin ace wates the near	ences, 2009) rules relative to r arest surface wat t apply.	orated comments the develop are not, act er source; the limited by	oment, classificivated by thin herefore, per an administr	fication at a sapplicat	nd/or ion. 0-502-

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B. GROUNDWATER AVAILABILITY CONSIDERATIONS, OAR 690-310-130, 400-010, 410-0070

B1.	Bas	sed upon available data, I have determined that groundwater* for the proposed use:
	a.	is over appropriated, is not over appropriated, $or \boxtimes$ cannot be determined to be over appropriated during any period of the proposed use. * This finding is limited to the groundwater portion of the over-appropriation determination as prescribed in OAR 690-310-130;
	b.	will not or will likely be available in the amounts requested without injury to prior water rights. * This finding is limited to the groundwater portion of the injury determination as prescribed in OAR 690-310-130;
	c.	\square will not or \square will likely to be available within the capacity of the groundwater resource; or
	d.	 will, if properly conditioned, avoid injury to existing groundwater rights or to the groundwater resource: i.
B2.	a.	Condition to allow groundwater production from no deeper than ft. below land surface;
	b.	Condition to allow groundwater production from no shallower than ft. below land surface;
	c.	Condition to allow groundwater production only from the groundwater reservoir between approximately ft. and ft. below land surface;
	d.	■ Well reconstruction is necessary to accomplish one or more of the above conditions. The problems that are likely to occur with this use and without reconstructing are cited below. Without reconstruction, I recommend withholding issuance of the permit until evidence of well reconstruction is filed with the Department and approved by the Groundwater Section.
		Describe injury —as related to water availability— that is likely to occur without well reconstruction (interference w/senior water rights, not within the capacity of the resource, etc):

B3. Groundwater availability remarks: Groundwater for the proposed use cannot be determined to be over-appropriated due to insufficient available data regarding rates of recharge and the current quantity of groundwater withdrawals from the aquifer system.

The proposed POA are or would be completed in sand and gravel of the Willamette Aquifer (Gannett and Caldwell, 1998). Proposed POA 1 (MARI 68155) produces from ~25 ft of black sand and gravel (some cemented) between ~185-207 ft below land surface (bls), which is overlain by fine-grained sediments to land surface (per the well log), generally classified as the Willamette Silt (Gannett and Caldwell, 1998). The other proposed POA, which have similar proposed construction to MARI 68155, are anticipated to produce water from the same or equivalent water-bearing zones. Some nearby wells (MARI 134 and MARI 304, for example) do produce relatively small quantities of water (<35 gpm) from thin (<10 ft thick) layers of sand within the Willamette Silt.

The nearest senior groundwater right producing from similar water-bearing zones as the proposed POA is MARI 143, authorized POA for Certificate 42114. MARI 143 is ~860 ft east of proposed POA 5. To assess the potential for injury to Certificate 42114 due to the proposed use, an analysis was conducted using the Theis (1935) equation for drawdown in a confined aquifer. Hydraulic parameters used for the analysis were derived from regional data and studies (Pumping Test Reports, Conlon et al., 2003, 2005; Iverson, 2002; McFarland and Morgan, 1996; Woodward et al., 1998) or are within a typical range of values for the given parameter within the hydrogeologic regime (Freeze and Cherry, 1979; Domenico and Mifflin, 1965). The applicant has noted in the Application that they have also submitted a Groundwater Registration Modification application (GR-MOD T-13221) which proposes to change the POA for Groundwater Registration GR-3351 to POA 1-5 as identified in this application. Per proposed GR-MOD T-13221, POA 1-5 would pump up to 0.25 cfs under GR-3351, in addition to the 0.45 cfs requested in this application. Therefore, the analysis used a combined rate of 0.7 cfs (~314 gpm) to assess potential injury to nearby senior users. Results indicate that the proposed use is not likely to interfere with MARI 143 such that a senior groundwater user would be prevented from receiving water to which they are legally entitled (see Theis Drawdown Analysis, attached).

Hydrographs of water levels in the nearby alluvial aquifer system over the past two decades do not indicate widespread or persistent declines (see Hydrograph - Spring, attached). However, there do appear to be substantial (~30 ft or more) seasonal

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fluctuations in water level based on observations from a nearby State Observation Well (see Hydrograph – MARI 308, attached). Additionally, well completion statistics from the area indicate slight trends of deeper well completions and lower initial reported static water levels over time (see Well Completion Statistics, attached). While there is not sufficient evidence to conclude that the proposed use would exceed the capacity of the resource, the Conditions specified in B1(d) and B2(c) are recommended to protect the resource and nearby senior users.

C. GROUNDWATER/SURFACE WATER CONSIDERATIONS, OAR 690-09-040

C1. **690-09-040** (1): Evaluation of aquifer confinement:

Well	Aquifer or Proposed Aquifer	Confined	Unconfined
1 - 5	Alluvium	\boxtimes	

Basis for aquifer confinement evaluation: Reported static water level in MARI 68155 was greater than 80 ft above the noted water-bearing zone, indicating confined conditions. Well completion statistics from this area indicate that most completed wells have initial reported static water levels above the first noted water-bearing zone, also indicating confined conditions (see Well Completion Statistics, attached). Based on the available evidence, the aquifer appears to be confined in this area.

C2. **690-09-040** (2) (3): Evaluation of distance to, and hydraulic connection with, surface water sources. All wells located a horizontal distance less than ½ mile from a surface water source that produce water from an unconfined aquifer shall be assumed to be hydraulically connected to the surface water source. Include in this table any streams located beyond one mile that are evaluated for PSI.

Well	SW #	Surface Water Name	GW Elev ft msl	SW Elev ft msl	Distance (ft)		Conn	ulically ected? ASSUMED	Potentia Subst. Int Assum YES	erfer.
1 – 5	1	Willamette River	~100-150	~63	~3,250 – 3,880	\boxtimes				\boxtimes
1 - 5	2	Deer Creek	~100-150	~169-157	~2,840 – 3,510	\boxtimes				\boxtimes

Basis for aquifer hydraulic connection evaluation: Water table maps of this area indicate that groundwater in the alluvial aquifer system is flowing toward and discharging into the Willamette River (SW 1) to the northwest of the proposed POA/POU (Woodward et al, 1998), an interpretation which is supported by the higher measured static groundwater elevations relative to the estimated surface water elevation for SW 1. SW 1 near this location may have incised into the water-bearing zone tapped by proposed POA 1 (MARI 68155) and which would be tapped by proposed POA 2-5. Based on the available evidence, the alluvial aquifer system in this area is hydraulically connected to SW 1 (Willamette River).

The proposed POA are also within 1 mile of Deer Creek (SW 2). Water table maps of this area indicate that static groundwater elevations in the alluvial aquifer system near SW 2 are within ~10 ft of the surface water elevations estimated for SW 2 (Woodward et al, 1998). Alluvial groundwater near SW 2 flows down-gradient to the northwest, through the area of the proposed POA, and into the Willamette River (SW 1). Although the reported static water elevation for proposed POA 1 (MARI 68155) is greater than 40 ft below the surface water elevations estimated for SW 2, the measurement reported for MARI 68155 was collected in August, when groundwater levels are anticipated to be temporarily depressed due to increased pumping and decreased recharge (see Hydrograph – MARI 308, attached). The preponderance of evidence indicates that SW 2 (Deer Creek) is hydraulically connected to the alluvial aquifer system.

Water Availability Basin the well(s) are located within: SW 1: WILLAMETTE R > COLUMBIA R - AB MOLALLA R SW 2: MILL CR > PUDDING R - AT MOUTH

C3a. **690-09-040 (4):** Evaluation of stream impacts for <u>each well</u> that has been determined or assumed to be **hydraulically connected and less than 1 mile** from a surface water source. Limit evaluation to instream rights and minimum stream flows that are pertinent to that surface water source, and not lower SW sources to which the stream under evaluation is tributary. Compare the requested rate against the 1% of 80% *natural* flow for the pertinent Water Availability Basin (WAB). If Q is not distributed by well, use full rate for each well. Any checked box indicates the well is assumed to have the potential to cause PSI.

Well	SW #	Well < 1/4 mile?	Qw > 5 cfs?	Instream Water Right ID	Instream Water Right Q (cfs)	Qw > 1% ISWR?	80% Natural Flow (cfs)	Qw > 1% of 80% Natural Flow?	Interference @ 30 days (%)	Potential for Subst. Interfer. Assumed?
1 – 5	1			MF182	1,500		3,830		>25%	\boxtimes
1 – 5	2						1.88	\boxtimes	<<25%	\boxtimes

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C3b. **690-09-040 (4):** Evaluation of stream impacts by total appropriation for all wells determined or assumed to be hydraulically connected and less than 1 mile from a surface water source. Complete only if Q is distributed among wells. Otherwise same evaluation and limitations apply as in C3a above.

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SW #	Qw > 5 cfs?	Instream Water Right ID	Instream Water Right Q (cfs)	Qw > 1% ISWR?	80% Natural Flow (cfs)	Qw > 1% of 80% Natural Flow?	Interference @ 30 days (%)	Potential for Subst. Interfer. Assumed?

Comments: 3a: To assess the potential interference with surface water due to the proposed use, a stream depletion analysis was conducted using the Hunt (2003) analytical model. Hydraulic parameters used for the analysis were derived from regional data and studies (Pumping Test Reports, Conlon et al., 2003, 2005; Iverson, 2002; McFarland and Morgan, 1996; Woodward et al., 1998) or are within a typical range of values for the given parameter within the hydrogeologic regime (Freeze and Cherry, 1979; Domenico and Mifflin, 1965). Results of the analysis indicate that the proposed use will primarily impact SW 1 (Willamette River), with depletions of SW 1 most likely exceeding 25 percent of the rate of well discharge within 30 days of continuous use (see Stream Depletion Analysis, attached). The high relative rate of stream depletion is attributed primarily to the small intervening thickness of fine-grained sediments between the alluvial aquifer system and SW 1 (Willamette River) (i.e. a highly efficient hydraulic connection) and the confined nature of the alluvial aquifer. Per OAR 690-009-0040(4)(d), the proposed use is assumed to have the potential to cause substantial interference (PSI) with SW 1 (Willamette River).

Additionally, the maximum rate of withdrawal under the proposed use (0.45 cfs) is greater than 1 percent (0.0188 cfs) of the natural streamflow that is equaled or exceeded 80 percent of time (1.88 cfs) for SW 2 (Deer Creek). Per OAR 690-009-0040(4)(c), the proposed use is assumed to have PSI with SW 2 (Deer Creek).

C4a. **690-09-040 (5):** Estimated impacts on **hydraulically connected surface water sources greater than one mile** as a percentage of the proposed pumping rate. Limit evaluation to the effects that will occur up to one year after pumping begins. This table encompasses the considerations required by 09-040 (5)(a), (b), (c) and (d), which are not included on this form. Use additional sheets if calculated flows from more than one WAB are required.

Non-Di	stributed	Wells											
Well	SW#	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
		%	%	%	%	%	%	%	%	%	%	%	%
Well Q	as CFS												
Interfere	ence CFS												
					-								
Distrib	uted Well	S											
Well	SW#	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
		%	%	%	%	%	%	%	%	%	%	%	%
Well Q	as CFS												
Interfere	ence CFS												
$(\mathbf{A}) = \mathbf{To}$	tal Interf.												
(B) = 80	% Nat. Q												
(C) = 1	% Nat. Q												
(D) = ($(\mathbf{A}) > (\mathbf{C})$	√	\checkmark	√	√	✓	\checkmark	√	\checkmark	√	√	✓	√
$(\mathbf{E}) = (\mathbf{A})$	/ B) x 100	%	%	%	%	%	%	%	%	%	%	%	%

(A) = total interference as CFS; (B) = WAB calculated natural flow at 80% exceed. as CFS; (C) = 1% of calculated natural flow at 80% exceed. as CFS; (D) = highlight the checkmark for each month where (A) is greater than (C); (E) = total interference divided by 80% flow as percentage.

Basis for impact evaluation: N/A

C4b.	690-09-040 (5) (b)	The potential to impair or detrimentally affect the public interest is to be determined by the Water
	Rights Section.	

C5. If properly conditioned, the surface water source(s) can be adequately protected from interference, and/or groundwater use
under this permit can be regulated if it is found to substantially interfere with surface water:
i. The permit should contain condition #(s);
ii. The permit should contain special condition(s) as indicated in "Remarks" below;
C6 SW / GW Remarks and Conditions:

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References Used:

Application File: G-18840

Transfer File: T-13029

Certificate: 42114, 93743

Groundwater Claim: GR-3351

Groundwater Registration Modification File: T-13221

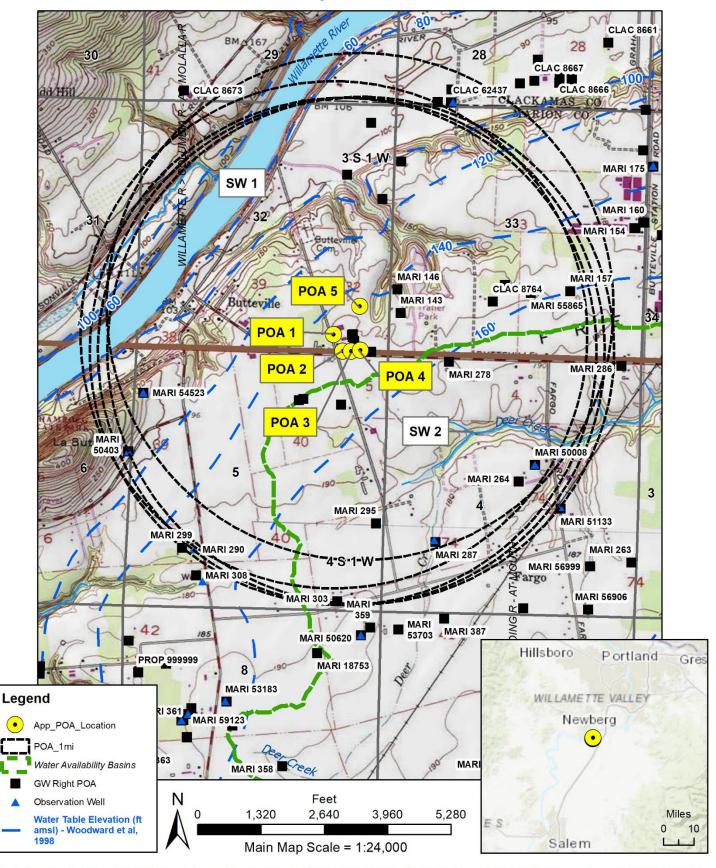
Pumping Test Reports: MARI 154, 160, 163, 172, 250, 348, 350, 358, 363, 53183, 53448, 54523

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- Theis, C.V., 1935. The relation between the lowering of the piezometric surface and the rate and duration of discharge of a well using groundwater storage, American Geophysical Union Transactions, vol. 16, p. 519-524.
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- Woodward, D.G., Gannett, M.W., and Vaccaro, J.J., 1998, Hydrogeologic framework of the Willamette Lowland aquifer system, Oregon and Washington: U.S. Geological Survey Professional Paper 1424-B, 82 p.

D. WELL CONSTRUCTION, OAR 690-200

D1.	Well #:	Logid:	
D2.	a. review of thb. field inspectc. report of CV	not appear to meet current well construction standards based upon: he well log; ction by	; ;
D3.	THE WELL constr	ruction deficiency or other comment is described as follows:	
D4.	Route to the Well (Construction and Compliance Section for a review of existing well construction.	

G-18840 Mayfield Farms, LLC



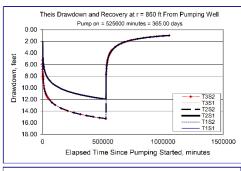
Service Layer Credits: Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), swisstopo, © OpenStreetMap contributors, and the GIS User Community Copyright:© 2013 National Geographic Society, i-cubed

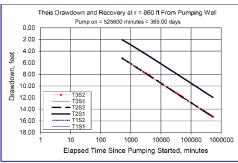
Theis Drawdown Analysis

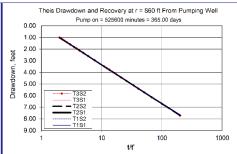
Theis Time-Drawdown Worksheet v.3.00
Calculates Theis nonequilibrium drawdown and recovery at any arbitrary radial distance, r, from a pumping well for 3 different T values and radial distance, r, from a pumping well for 3 different T values and 2 different S values Written by Karl C. Wozniak September 1992. Last modified December 30, 2014

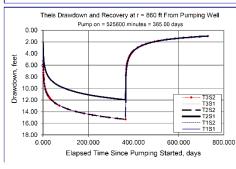
Input Data:	Var Name	Scenario 1	Scenario 2	Scenario 3	Units	
Total pumping time	t		365		d	
Radial distance from pumped well:	r		860.00		ft	Q conversions
Pumping rate	Q		314.0		gpm	314.00 gpm
Hydraulic conductivity	K	66.000	66.000	66.000	ft/day	0.70 cfs
Aquifer thickness	b		50		ft	41.98 cfm
Storativity	S_1		0.00100			60,449.20 cfd
	S_2		0.00010			1.39 af/d
Transmissivity Conversions	T_f2pd	3,300	3,300	3,300	ft2/day	
	T_ft2pm	2.2917	2.2917	2.2917	ft2/min	
	T apapft	24,684	24.684	24.684	gpd/ft	1

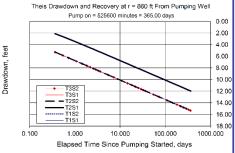
Use the Recalculate button if recalculation is set to manua

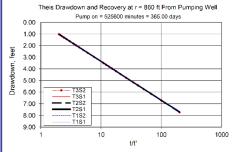




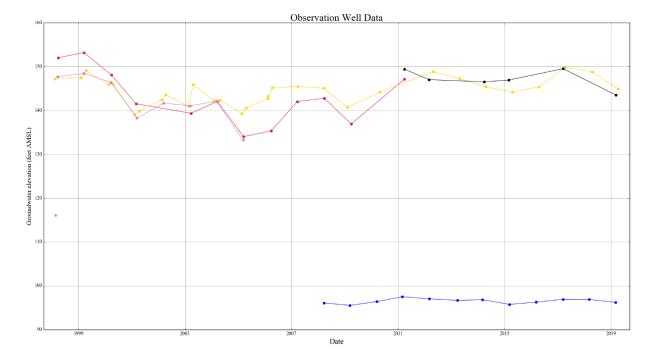




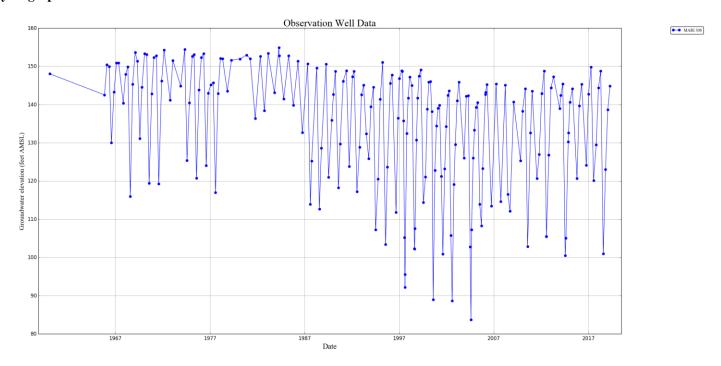




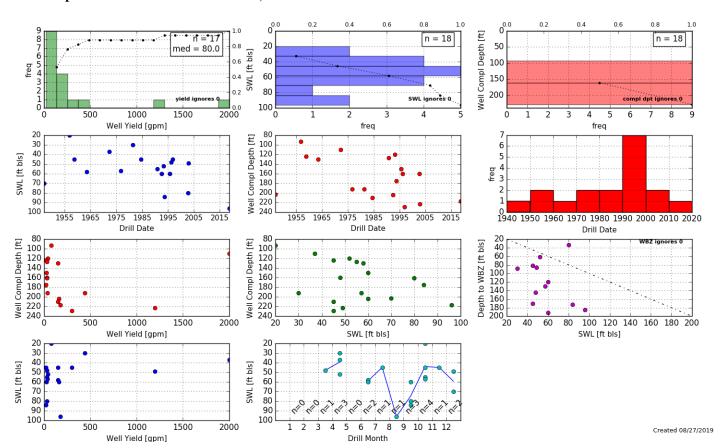
Hydrograph - Spring



Hydrograph - MARI 308



Well Completion Statistics - 3S/1W-32 & 33, 4S/1W-3 & 4



Water Availability Tables

Water Availability Analysis

Detailed Reports

WILLAMETTE R > COLUMBIA R - AB MOLALLA R WILLAMETTE BASIN

Water Availability as of 8/27/2019

Watershed ID #: 182 (Map)

Date: 8/27/2019

Water Availability Calculation Consumptive Uses and Storages Instream Flow Requirements Reservations

Water Rights Watershed Characteristics

Water Availability Calculation

Monthly Streamflow in Cubic Feet per Second Annual Volume at 50% Exceedance in Acre-Feet

Month	Natural Stream Flow	Consumptive Uses and Storages	Expected Stream Flow	Reserved Stream Flow	Instream Flow Requirement	Net Water Available
JAN	21,400.00	2,300.00	19,100.00	0.00	1,500.00	17,600.00
FEB	23,200.00	7,490.00	15,700.00	0.00	1,500.00	14,200.00
MAR	22,400.00	7,260.00	15,100.00	0.00	1,500.00	13,600.00
APR	19,900.00	6,920.00	13,000.00	0.00	1,500.00	11,500.00
MAY	16,600.00	4,260.00	12,300.00	0.00	1,500.00	10,800.00
JUN	8,740.00	1,980.00	6,760.00	0.00	1,500.00	5,260.00
JUL	4,980.00	1,810.00	3,170.00	0.00	1,500.00	1,670.00
AUG	3,830.00	1,650.00	2,180.00	0.00	1,500.00	677.00
SEP	3,890.00	1,400.00	2,490.00	0.00	1,500.00	992.00
OCT	4,850.00	758.00	4,090.00	0.00	1,500.00	2,590.00
NOV	10,200.00	891.00	9,310.00	0.00	1,500.00	7,810.00
DEC	19,300.00	973.00	18,300.00	0.00	1,500.00	16,800.00
ANN	15,200,000.00	2,250,000.00	13,000,000.00	0.00	1,090,000.00	11,900,000.00

Water Availability Analysis

Detailed Reports

MILL CR > PUDDING R - AT MOUTH WILLAMETTE BASIN

Water Availability as of 8/27/2019

Watershed ID #: 30200901 (Map)

Date: 8/27/2019

Exceedance Level: 80% >

Time: 1:36 PM

Water Availability Calculation Consumptive Uses and Storages Instream Flow Requirements Reservations

Water Rights Watershed Characteristics

Water Availability Calculation

Monthly Streamflow in Cubic Feet per Second Annual Volume at 50% Exceedance in Acre-Feet

Month	Natural Stream Flow	Consumptive Uses and Storages	Expected Stream Flow	Reserved Stream Flow	Instream Flow Requirement	Net Water Available
JAN	39.20	9.74	29.50	0.00	0.00	29.50
FEB	53.90	9.89	44.00	0.00	0.00	44.00
MAR	38.40	9.47	28.90	0.00	0.00	28.90
APR	27.60	7.09	20.50	0.00	0.00	20.50
MAY	13.70	5.70	8.00	0.00	0.00	8.00
JUN	8.72	7.01	1.71	0.00	0.00	1.71
JUL	3.79	10.80	-6.96	0.00	0.00	-6.96
AUG	2.09	8.74	-6.65	0.00	0.00	-6.65
SEP	1.88	4.78	-2.90	0.00	0.00	-2.90
OCT	2.39	1.25	1.14	0.00	0.00	1.14
NOV	6.05	7.23	-1.18	0.00	0.00	-1.18
DEC	25.90	9.57	16.30	0.00	0.00	16.30
ANN	30,000.00	5,500.00	25,300.00	0.00	0.00	25,300.00

Version: 05/07/2018

Exceedance Level: 80% V

Time: 1:36 PM

$Stream\ Depletion\ Analysis-SW\ 1\ (Willamette\ River)$

Application type: G	Depiction Analysis – 5 W	((((Maniette Kivei)						
Application number: Well number: Stream Number: Pumping rate (cfs): Pumping duration (days): Pumping start month number (3=March) Distance from well to stream Aquifer transmissivity Aquifer storativity Aquifer storativity Aquitard vertical hydraulic conductivity Aquitard saturated thickness ba 120.0 12		Application type			G			
Vell number: 1								
Stream Number: 1 0.45								
Pumping rate (cfs): Pumping duration (days): Pumping start month number (3=March) Parameter Symbol Scenario 1 Scenario 2 Scenario 3 Distance from well to stream a 3880.0 3565 3250 ft Aquifer transmissivity T 2000 3300.0 4300.0 ft2/day Aquifer storativity S 0.001 0.0005 0.0001 - Aquitard vertical hydraulic conductivity Kva 0.003 0.005 0.01 ft/day Aquitard saturated thickness ba 120.0 120.0 120.0 ft Aquitard thickness below stream babs 4.0 4.0 4.0 ft Aquitard specific yield Sya 0.2 0.2 0.2 - Stream width ws 650.0 650.0 650.0 ft Stream depletion for Scenario 2: Days 10 30 60 90 120 150 180 210 240 270 300 330 360 Depletion (%) 30 35 35 35 35 36 36 36 36								
Pumping duration (days): Pumping start month number (3=March) Parameter Symbol Scenario 1 Distance from well to stream Aquifer transmissivity T Distance from well to stream Aquifer storativity S Distance on the stream of								
Pumping start month number (3=March) 1.0								
Parameter Symbol Scenario 1 Scenario 2 Scenario 3 Units								
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Aquifer transmissivity T 2000 3300.0 4300.0 ft2/day Aquifer storativity S 0.001 0.0005 0.0001 - Aquitard vertical hydraulic conductivity Kva 0.003 0.005 0.01 ft/day Aquitard saturated thickness ba 120.0 120.0 120.0 ft Aquitard thickness below stream babs 4.0 4.0 4.0 ft Aquitard specific yield Sya 0.2 0.2 0.2 - Stream width ws 650.0 650.0 650.0 ft Stream depletion for Scenario 2: Days 10 30 60 90 120 150 180 210 240 270 300 330 360 Depletion (%) 30 35 35 35 35 36 36 36 36 36 37 37 37 Depletion (cfs) 0.13 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.16	Distance fron		-					
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Stream width ws 650.0 650.0 650.0 ft Stream depletion for Scenario 2: Days 10 30 60 90 120 150 180 210 240 270 300 330 360 Depletion (%) 30 35 35 35 35 36 36 36 36 36 36 37 37 37 Depletion (cfs) 0.13 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.16					_			
Stream depletion for Scenario 2: Days 10 30 60 90 120 150 180 210 240 270 300 330 360 Depletion (%) 30 35 35 35 35 36 36 36 36 36 37 37 37 Depletion (cfs) 0.13 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.16		•	-					
Days 10 30 60 90 120 150 180 210 240 270 300 330 360 Depletion (%) 30 35 35 35 35 36 36 36 36 36 37 37 37 Depletion (cfs) 0.13 0.16 0.16 0.16 0.16 0.16 0.16 0.16 0.16	Stream width	h	WS	650.0	650.0	650.0	ft	
1.0	Depletion (%) 30	30 60 90 12 35 35 35 35	20 150 5 36) 180 36	210 240 36 36	36 37	37 37	
Scenario 3 Scenario 2 0.40 0.35 0.30 0.25	Hunt (2003) transient stream depletion model							
— Scenario 2 0.40 Scenario 1 0.35 0.30 9 0.25 9								
0.8 Scenario 1 0.35 0.30 0.30 0.25 10	arg				_		11040	
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Stream Depletion Analysis – SW 2 (Deer Creek)

Application type:	G
Application number:	18840
Well number:	1
Stream Number:	2
Pumping rate (cfs):	0.45
Pumping duration (days):	365.0
Pumping start month number (3=March)	1.0

Parameter	Symbol	Scenario 1	Scenario 2	Scenario 3	Units
Distance from well to stream	a	2840	2840	2840	ft
Aquifer transmissivity	T	500.0	3300.0	4300.0	ft2/day
Aquifer storativity	S	0.001	0.0003	0.0001	-
Aquitard vertical hydraulic conductivity	Kva	0.01	0.005	0.001	ft/day
Aquitard saturated thickness	ba	140.0	140.0	140.0	ft
Aquitard thickness below stream	babs	140.0	140.0	140.0	ft
Aquitard specific yield	Sya	0.2	0.2	0.2	2
Stream width	ws	10.0	10.0	10.0	ft

Stream depletion for Scenario 2: Days 120 150 240 10 30 60 180 210 270 300 330 360 Depletion (%) 0 0 0 0 0 0 0 0 0 0 0 0 0 Depletion (cfs) 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

